

Memorandum

To: Lauren Kemp
Construction Liaison
District 11 Environmental

Date: September 13, 2007
File: 11-SD-905-9.2/18.0
EA 11-274801

From: DEPARTMENT OF TRANSPORTATION
DIVISION OF ENGINEERING SERVICES
Geotechnical Services
Office of Geotechnical Design - South

Subject: Proposed Slurry Basins at 905 and Caliente Avenue

Introduction

Pursuant to your request an investigation has been conducted of the proposed slurry basin site as shown on the attached slurry plan sheet. The basins are intended to temporarily store the slurry produced from the grinding of concrete pavement. The investigation included site reconnaissance, review of the project Geotechnical Design Report (GDR) to construct the SR-905 Freeway and other archived documents, four shallow soil borings by hand auger, and percolation testing. This report is intended to convey information relevant to the design of the proposed slurry basins. Additional pertinent data concerning the slurry basin site can be found in the SR-905 Freeway GDR.

Site Description

In it's current condition the proposed slurry basin site is fallow farmland that gently slopes a few meters in elevation from northwest to southeast. The site is bounded on the north by existing SR-905 and on the south by broad topographic swale.

Exploration and Testing

Numerous soil borings and extensive geologic mapping was previously completed along the SR-905 alignment during preparation of the project GDR. Four of those borings (BB4, BB5, BB6, and BB7) were located relatively close to the proposed slurry basin site and may be considered representative of the subsurface conditions underlying the proposed basins. Copies of the field boring logs are attached to this report. The locations of borings BB5 and BB6 are depicted on the attached plan sheet.

Four shallow hand auger borings (PT1 through PT4) were conducted specifically for this investigation. The location of each boring is shown on the attached slurry basin plan sheet. The

depth of the shallow borings was 1.6 meters. The depth of each boring was selected to approximate the depth of basin grading.

Percolation testing was conducted in the four shallow hand auger borings in accordance with CTM 750. The results of the percolation test are attached to this report. The following percolation rates were recorded:

PT1	63.1 min/in
PT2	12.4
PT3	30.2
PT4	40.3

Soil gradation testing was not performed, as the site soils are formational sandstone, siltstone, and claystone.

Site Geology

The soil borings and geologic mapping clearly detail the subsurface conditions. A surficial layer of slightly organic clayey silt topsoil mantles the proposed slurry basin site. Soft/weak siltstone and fine grain sandstone of the Otay Formation underlie the topsoil. This flat lying sedimentary formation contains interbedded claystone and slightly indurated layers of somewhat harder formation.

Groundwater

No groundwater was encountered in the shallow hand auger borings conducted within the proposed slurry basin site. No Groundwater was encountered in any of the nearby borings conducted for the 905 project GDR and drilled to maximum depths of about 8.2 meters. The Department of Water Resources Bulletin 106-2 describes the general occurrence of groundwater in the area as being at depths greater than 100 feet. This generalization was confirmed by relatively deep borings conducted for bridge foundation exploration on the SR-905 project.

Recommendations

The site is well suited to accommodate the proposed slurry basins without the need for an impermeable basin liner. The site soils are easily rippable by heavy grading equipment. Soil percolation rates are relatively low. The prevailing geologic formation does not readily store or convey subsurface water. There are no aquifers in the project area. The proposed basins will not leach slurry into groundwater resources. Due to the hydrogeologic behavior of the surrounding sedimentary formation, an average percolation rate of roughly 44 min/in will govern basin behavior. However, the precipitation of slurry cake will quickly render the basin floor essentially impermeable.

The graded soil berms that form the sides of the proposed slurry basins will be significantly more permeable than the undisturbed native formation. To minimize seepage, the

soil berms should be placed to a relative compaction of 90% with a minimum top width of 3 meters and a side slope gradient of 1:2 (vertical to horizontal). Mild water seepage through the berms should be anticipated, however, such seepage is not necessarily problematic. Berm seepage rates may be insufficient to produce surface flow, and any concentrated seepage may be effectively managed. The basin side of the berms may be lined to reduce the seepage rate. If utilized, the impermeable liner should extend a minimum of two meters onto the floor of the basins. For optimum results seams in the lining should be minimized. Lining of the basin floors may be considered superfluous since the precipitation of slurry cake will quickly render the basin floor essentially impermeable.

The width and relative compaction of the graded soil berms may be substantially reduced if the slurry basins are constructed with a competent impermeable liner. In addition to the design of basin liners, responsible staff outside OGDS should provide the design of minimally dimensioned and compacted earth berms.

OGDS staff is available to provide further site information and recommendations. If you have additional questions or require clarification please contact Brian Hinman at (office) 858 467-4051 or (mobile) 858 705-1344.



Brian Hinman
Senior Transportation Engineer
Office of Geotechnical Design - South



cc: Abbas Abghari
OGDS2 Files

Attachments

PERCOLATION TEST

GEOTECHNICAL DESIGN SOUTH-2

PROJECT		SITE LAYOUT/NOTES
EA #		
DATE	9/7/07	
TEST MADE BY	ECG	
AMBIENT TEMP		
WEATHER COND'N	SUNNY	
SOAKING PERIOD	24 hrs	
TYPE OF SOIL	SAND & SILTSTN	

HOLE NO.	PT-1	PT-2	PT-3	PT-4				
HOLE DEPTH (ft)	5'6"	5'1"	5'1"	5'2"				
HOLE DIA. (in)	6"	6"	6"	6"				
READING	TIME	TIME	TIME	TIME	TIME	TIME	TIME	TIME
8" - 7"	12'10"	2'25"	6'25"	8'31"				
8" - 7"	15'13"	3'24"	7'33"	11'36"				
8" - 7"	18'09"	3'31"	8'28"	13'08"				
8" - 7"	18'51"	3'45"	9'08"	13'01"				
8" - 7"	19'12"	3'59"	9'25"	12'02"				
8" - 7"	19'25"	2'51"	9'38"	12'35"				
	20'19"							
AVERAGE (LAST 3) = R = _____ min/in	19'39"	3'51"	9'24"	12'33"				
P =	63.1	12.4	30.2	40.3				

CALCULATION:

AVE. OF LOW RATES: $P_{AVE} \approx 44 \text{ min/in}$

1) Correction Factor ©

$$C = n[1 - (O/D)^2] + (I/D)^2 = 0.536111$$

Porosity of the pea gravel,

$$n = 0.3$$

Inside diameter of perforated pipe in inches,

$$I = 4$$

Outside diameter of perforated pipe in inches,

$$O = 5$$

Actual diameter of percolation test hole in inches,

$$D = 6$$

2) Conversion factor (K)

$$K = 0.27 + 8.7/D$$

$$K = 1.72$$

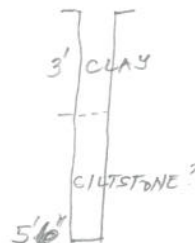
3) Equivalent Unlined 12-inch Diameter Percolation Rate (P)

$$P = K.R/C$$

$$P = \text{_____ min/in} \quad 0$$

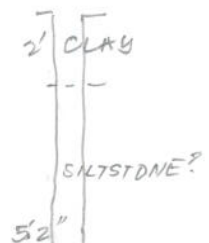
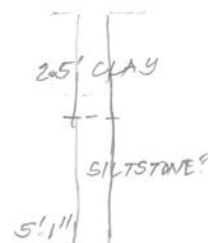
PT-1

PT-2



PT-3

PT-4



Continued on Page 2 of 2

Boring No. BB-5		SOILS AND GEOLOGICAL EXPLORATION LOG						Page 1 of 2	
Date Started 4/8/2004		Date Ended 4/8/2004		Geologist & Crew M Jandal		Project Name SR-905		EA 11-091821	
Top Hole Elev. 165 m		Location Offset & Station: Ctrl. 132+20				Purpose of work GDR Investigation		Dist - Co. - Rte. - PM 11-SD-905	
Boring Depth (ft)	Boring Depth (meter)	Sample Number	Groundwater Level		Depth to Water	Total Hole Depth	SPT Blows per .3 meter	Graphic Log	Lithological Descriptions
			Date Measured						
			SPT	CA Modified	Coring	Total Recovery	% Recovery	RQD	Lab Work Assigned
1									
2									
3	1								
4									
5									
6	2		27						
			28						
			23						
8									
9									
10	3								
11			25						
			60						
			50						
12									
13	4								
14									
15									
16	5		12						
17									
18									
19									
20	6								

SM Silty Sand, very dense, light gray and reddish brown, slightly moist with gravel and calcareous nodules (Terrace Deposit)

- at 2 m hit a layer of gravel

GP Poorly Graded Sandy Gravel, very dense, reddish brown (Terrace Deposit)

- refusal, 12 blows for 25 mm of penetration (the hammer was bouncing upon impact)

Continued on Page 2 of 2

Boring No. BB-6		SOILS AND GEOLOGICAL EXPLORATION LOG						Page 1 of 2	
Date Started		Date Ended		Geologist & Crew		Project Name		EA	
4/7/2004		4/7/2004		A. Lari		SR-905		11-091821	
Top Hole Elev.		Location				Purpose of work		Dist - Co. - Rte. - PM	
160 m		Offset & Station: 10 m Rt. 134+00				Ref Line E5		GDR Investigation	
Groundwater Level		Date Measured		Depth to Water		Drilling Method		Drilling Equipment	
						Wet core drilling		Truck Mounted Rig (CS 500)	
Total Hole Depth		SPT Blows per .3 meter		SPT total		Lithological Descriptions			
Boring Depth (ft)	Boring Depth (meter)	Sample Number	SPT	CA Modified	Coring	Total Recovery	% Recovery	RQD	Lab Work Assigned
Graphic Log									
1. Group Name									
2. Group Symbol									
3. Consistency/Relative Density									
4. Color									
5. Moisture									
6. Partical Size & Shape									
7. Gradation									
8. Plasticity									
9. Structure									
10. Cementation									
11. Organics									
12. Fill Material									
13. Other									
1									
2									
3	1								
4									
5			32						
6			50						50+
7			50						
8	2								
9									
10									
11									
12									
13	3		46						
14			50						50+
15									
16									
17	4								
18									
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Boring No. BB-6		SOILS AND GEOLOGICAL EXPLORATION LOG						Page 2 of 2	
Date Started		Date Ended		Geologist & Crew		Project Name		EA	
4/7/2004		4/7/2004		A. Lari		SR-905		11-091821	
Top Hole Elev.		Location				Purpose of work		Dist - Co. - Rte. - PM	
160 m		Offset & Station: 10 m Rt. 134+00				Ref Line E-5		GDR Investigation	
Groundwater Level		Date Measured		Depth to Water		Drilling Method		Drilling Equipment	
						Wet core drilling		Truck Mounted Rig (CS 500)	
Total Hole Depth		SPT Blows per .3 meter		SPT total		Lithological Descriptions			
SPT		CA Modified		Coring		Total Recovery		% Recovery	
RQD		Lab Work Assigned		15 meter		1. Group Name		8. Plasticity	
						2. Group Symbol		9. Structure	
						3. Consistency/Relative Density		10. Cementation	
						4. Color		11. Organics	
						5. Moisture		12. Fill Material	
						6. Partical Size & Shape		13. Other	
						7. Gradation			
21	7								
22									
23									
24									
25		30							
26	8								
27									
28									
29									
30	9								
31									
32									
33									
34	10								
35									
36									
37									
38	11								
39									
40									
41									
42	12								
43									
44									
45									

SP Poorly Graded Sand, very dense, dark gray (Terrace Deposit)

- hit rock, slow drilling

GW Well Graded gravel, very dense, dark gray, refusal, 30 blows for 75 mm of penetration (Terrace Deposit)

End of boring at 8.2 meters

Ground water was not encountered during the drilling operations.

